



Forum 1: Planning and building

Mounting and substructure and their importance for the power plant?



10TH/11TH March, Paris, France

© Dr. Zapfe GmbH 2011





Overview of topics:

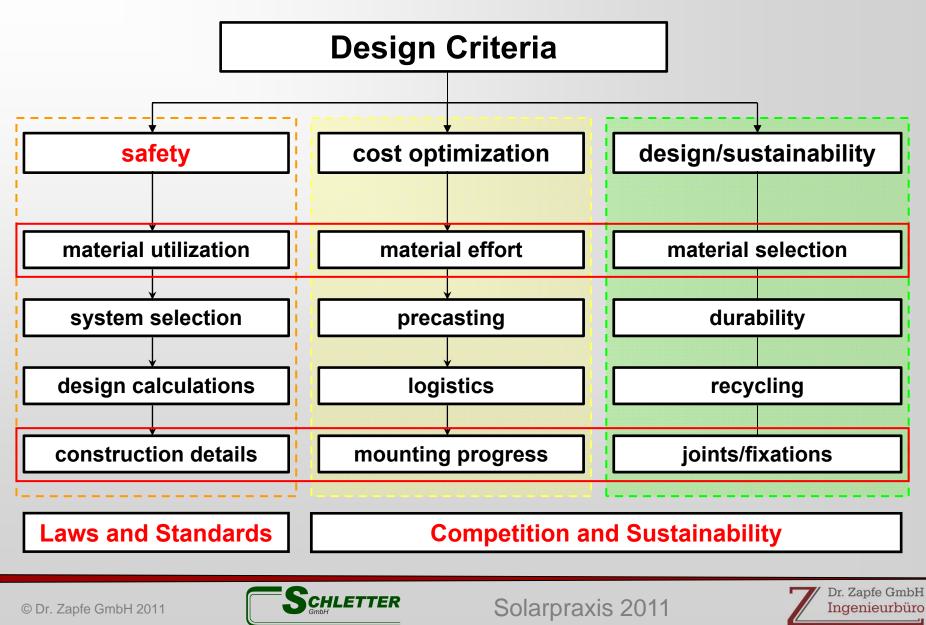
- 1. Introduction
- 2. Load evaluation
- 3. Design calculations
- 4. Decision criteria for a substructure selection
- 5. Foundation concepts
- 6. Mounting progress of foundation concepts
- T. Summary

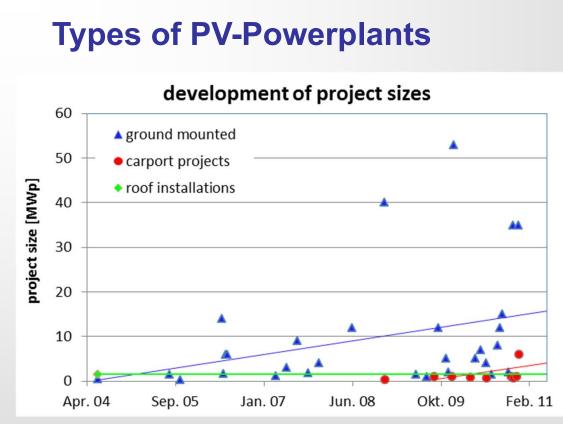






1. Introduction







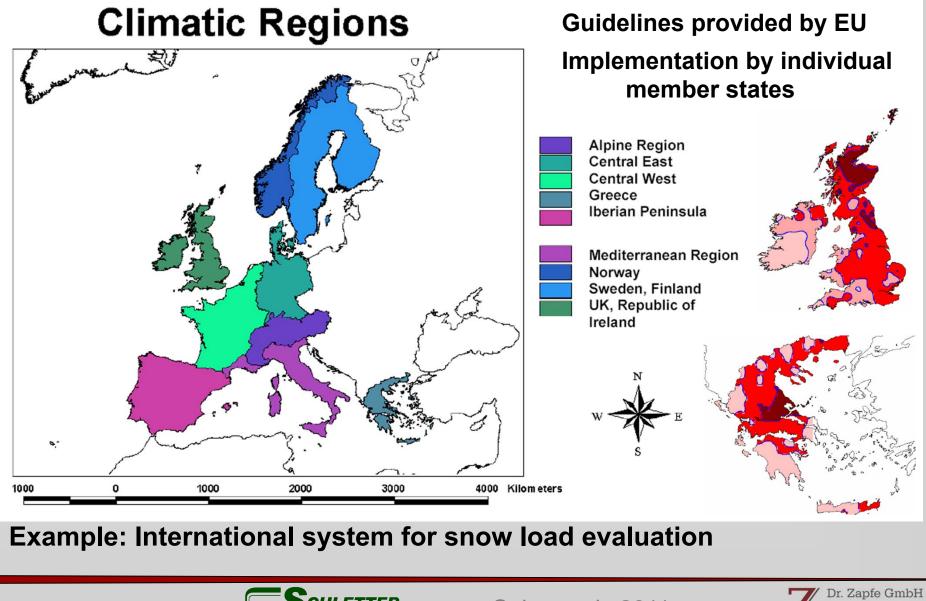








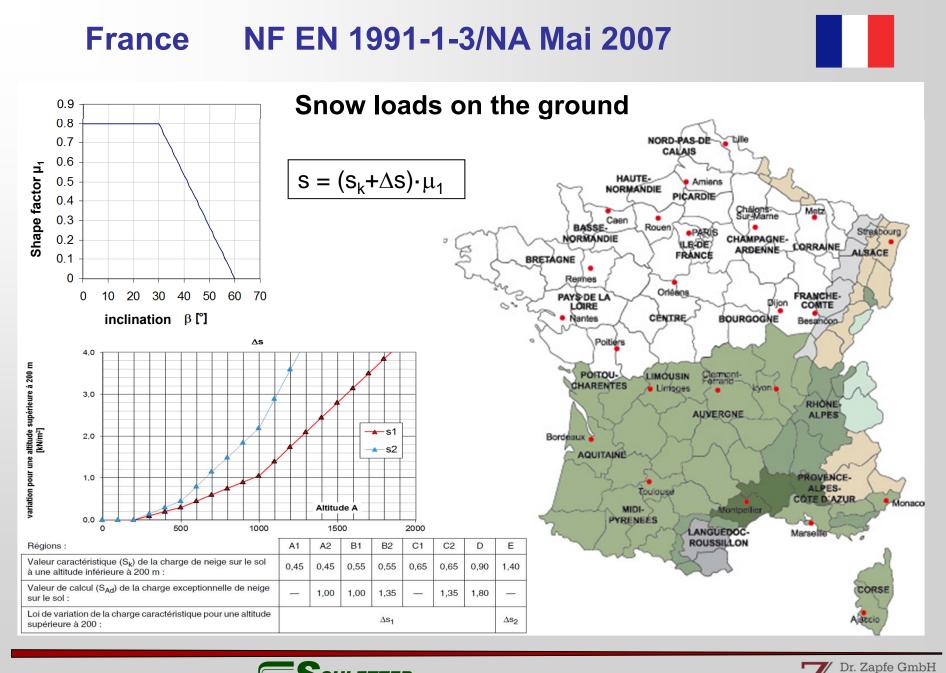
2. load actions



© Dr. Zapfe GmbH 2011







Solarpraxis 2011

Ingenieurbüro



European wind zone map according to Eurocode 1



Basis:

Measurements (188 in D)

10-minutes median in10 m height above groundthat occurs once every 50years

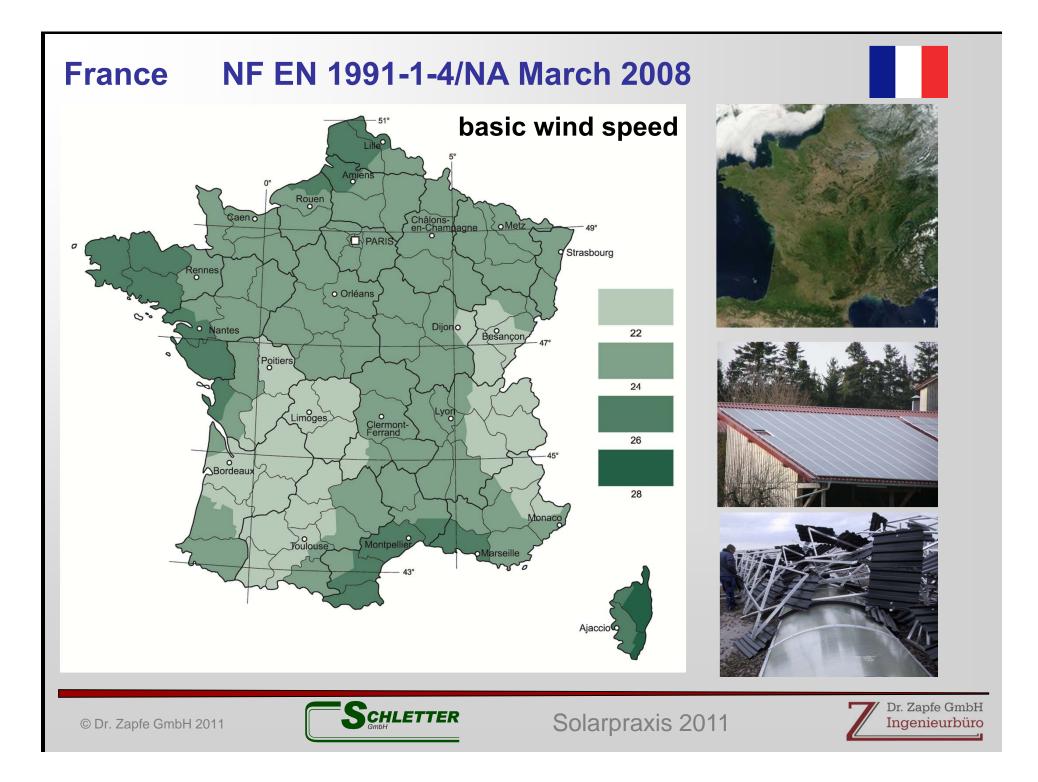
observation period: 40-107 years

contains no gusts

applicable for flat, even terrain







Terrain categories according to Eurocode 1

Terrain category 0

Sea, coastal area exposed to the open sea

Terrain category I (II in France)

Lakes or area with negligible vegetation and without obstacles

Terrain category II (III a in France)

Area with low vegetation such as grass and isolated obstacles (trees, buildings) with separations of at least 20 obstacle heights

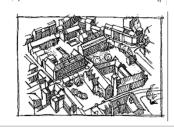


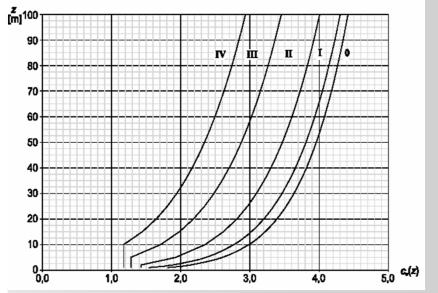
Terrain category III (III b in France)

Area with regular cover of vegetation or buildings or with isolated obstacles with separations of maximum 20 obstacle heights (such as villages, suburban terrain, permanent forest)

Terrain category IV

Area in which at least 15 % of the surface is covered with buildings and their average height exceeds 15 m





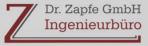
Basis:

- $q_b = \frac{1}{2} \cdot \rho \cdot v^2$ (basic pressure)
- ρ weight of air (1,25 kg/m²)

Peak velocity pressure

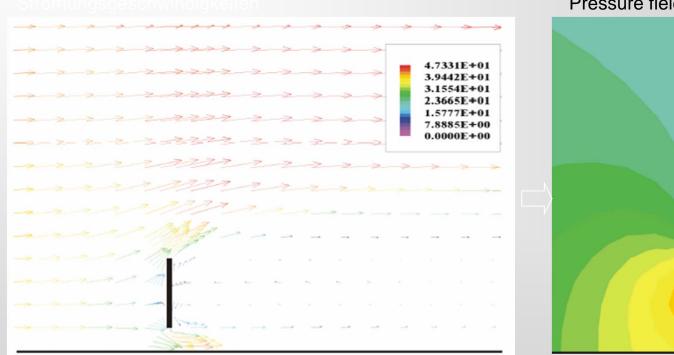
$$q_{b}(z) = C_{e}(z) \cdot q_{b}$$





Aerodynamic characteristics

Pressure field if a vertical flow impacts the screen



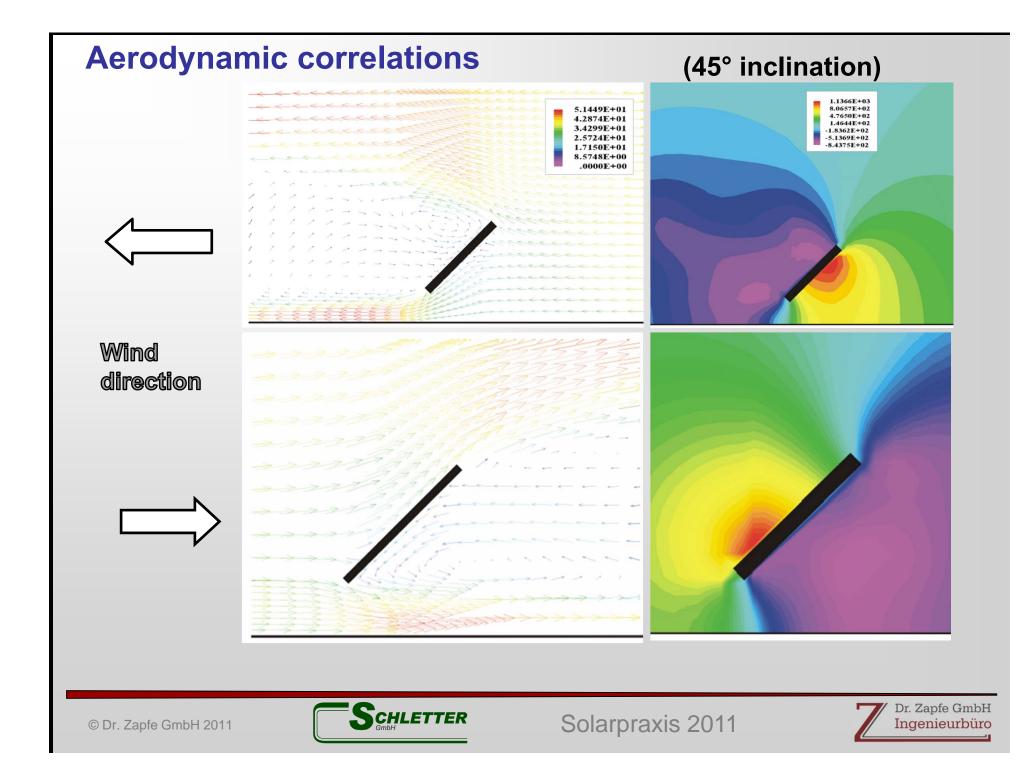
Pressure field (qualitative)

Source: Final report 0327229 A, patronized by the Federal Ministry of Economy and Technology



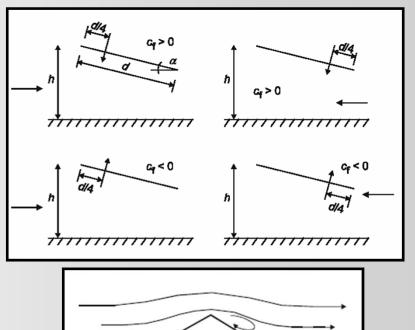




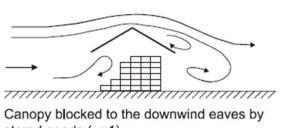


Pressure and Force Coefficients (DIN EN 1991-1-4)

				A C	s c _{p,net} <u>↓</u> ↑ b/10 b
Dest		Overall Force	↔ d/10 ←	B d/10 ↔ d] <u>∓</u> ↓
Roof angle α	Blockage φ	Coefficients _{Cf}	Zone A	Zone B	Zone C
0°	Maximum all φ	+ 0,2	+ 0,5	+ 1,8	+ 1,1
	Minimum $\phi = 0$	- 0,5	- 0,6	- 1,3	- 1,4
	Minimum φ = 1	- 1,3	- 1,5	- 1,8	- 2,2
5°	Maximum all ϕ	+ 0,4	+ 0,8	+ 2,1	+ 1,3
	Minimum φ = 0	- 0,7	- 1,1	- 1,7	- 1,8
	Minimum φ = 1	- 1,4	- 1,6	- 2,2	- 2,5
10°	Maximum all ϕ	+ 0,5	+ 1,2	+ 2,4	+ 1,6
	Minimum $\varphi = 0$	- 0,9	- 1,5	- 2,0	- 2,1
	Minimum φ = 1	- 1,4	- 2,1	- 2,6	- 2,7
15°	Maximum all ϕ	+ 0,7	+ 1,4	+ 2,7	+ 1,8
	Minimum $\varphi = 0$	- 1,1	- 1,8	- 2,4	- 2,5
	Minimum φ = 1	- 1,4	- 1,6	- 2,9	- 3,0
20°	Maximum all ϕ	+ 0,8	+ 1,7	+ 2,9	+ 2,1
	Minimum φ = 0	- 1,3	- 2,2	- 2,8	- 2,9
	Minimum φ = 1	- 1,4	- 1,6	- 2,9	- 3,0
25°	Maximum all ϕ	+ 1,0	+ 2,0	+ 3,1	+ 2,3
	Minimum φ = 0	- 1,6	- 2,6	- 3,2	- 3,2
	Minimum φ = 1	- 1,4	- 1,5	- 2,5	- 2,8
30°	Maximum all ϕ	+ 1,2	+ 2,2	+ 3,2	+ 2,4
	Minimum φ = 0	- 1,8	- 3,0	- 3,8	- 3,6
	Minimum φ = 1	- 1,4	- 1,5	- 2,2	- 2,7



Empty, free-standing canopy (ϕ =0)



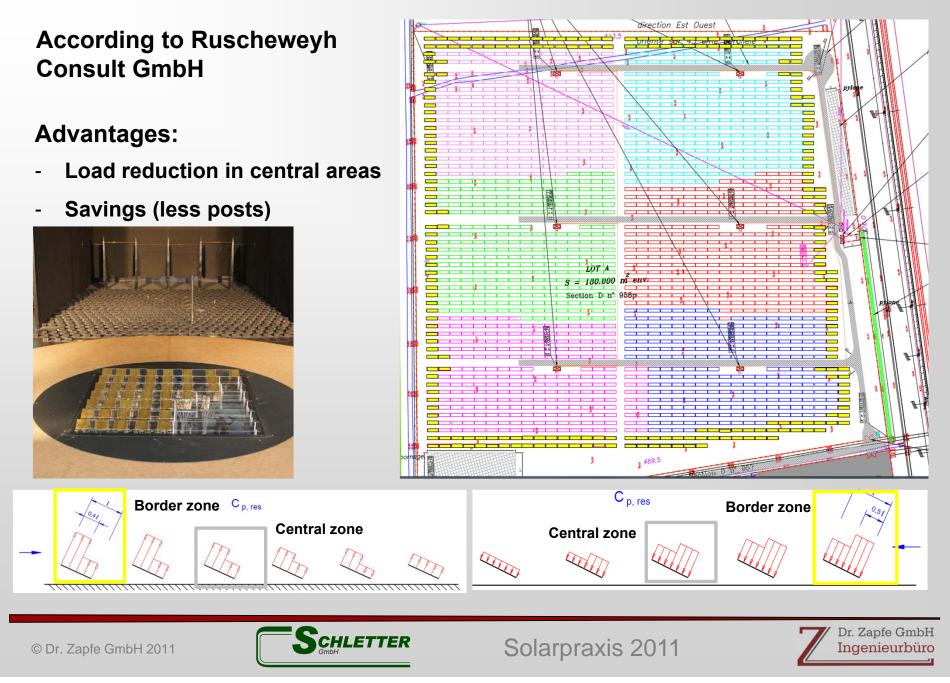
stored goods (q=1)

© Dr. Zapfe GmbH 2011

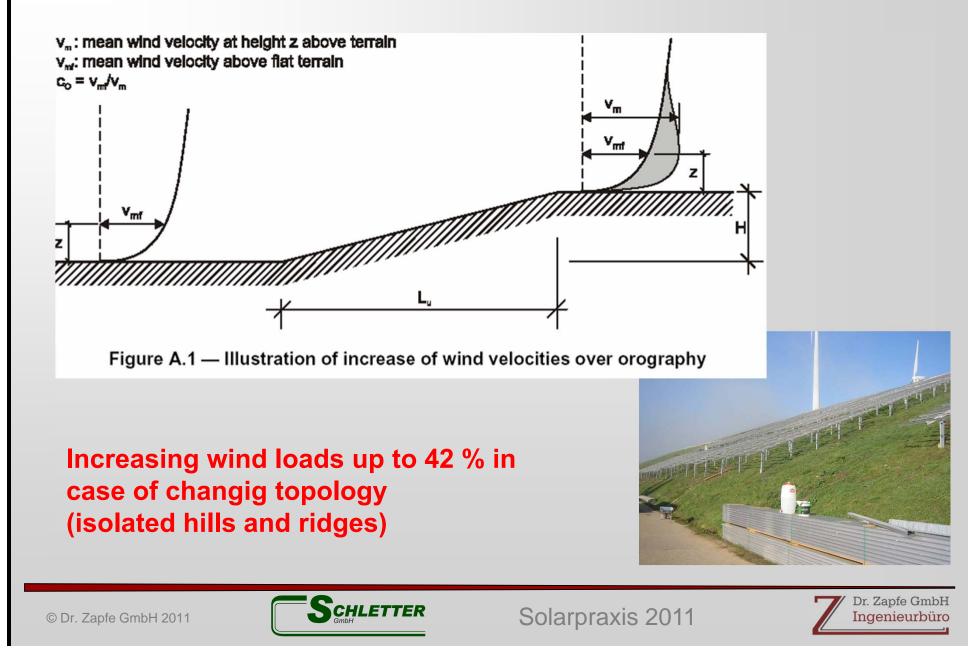




Pressure Coefficients from Windtunnel tests



Increasing wind loads in case of isolated hills and ridges



3. Design calculations for PV systems

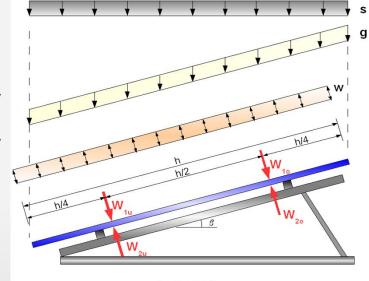
Load combinations

LC 1: 1,35·g + 1,5·s + 0,6·1,5·w

LC 2: 1,35·g + 0,5·1,5·s + 1,5·w

LC 3: 0,9·g + 1,5·w

(uplift)



$b = h \cdot \cos \beta$

Verifications

- tilting
- dragging

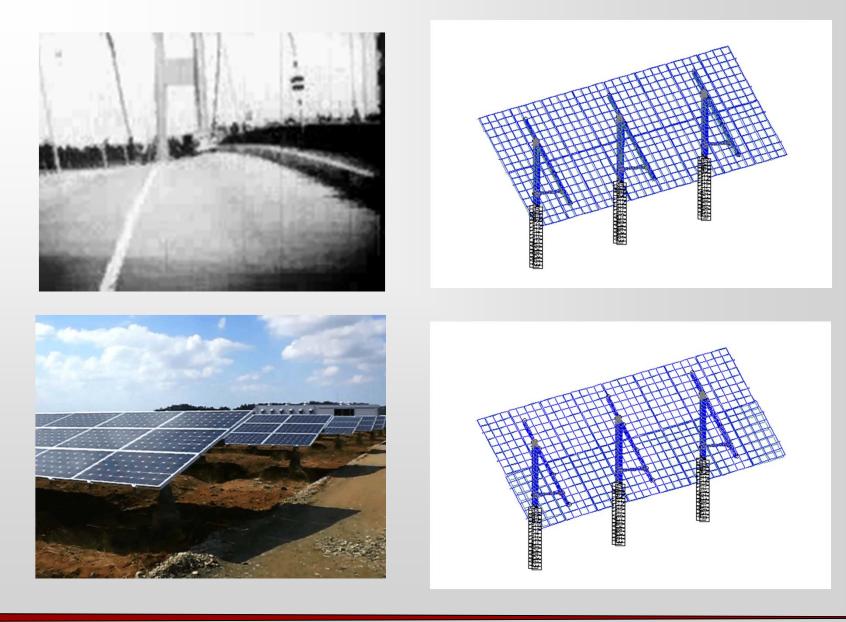
• uplift



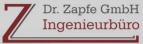


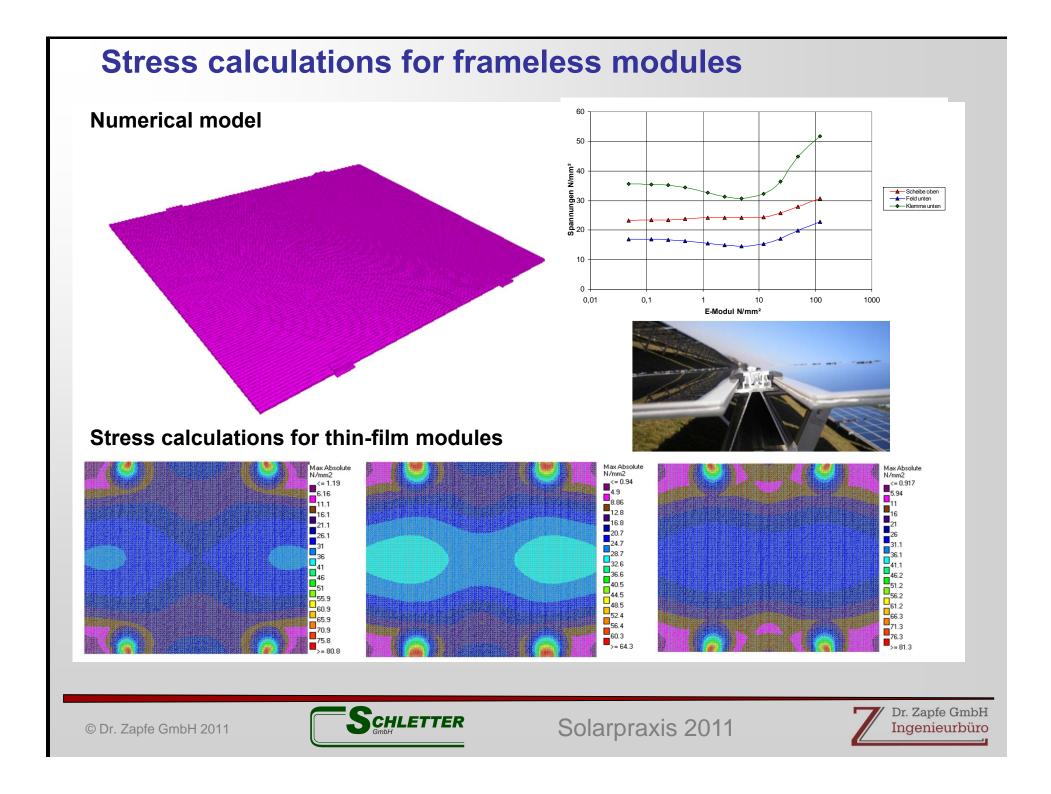


Wind-induced vibrations / seismic design









4. Decision criteria for substructure selection

Material (dimensioning acc. to basic material standards)

Aluminum

- + low selfweight
- + shaping by extrusion process
- + easy to install (tolerance equalization)
- + remaining value
- floating material prices
- low youngs modulus

Steel

- + availability / well-proven solutions
- corrosion protection
- mounting effort
- high weight

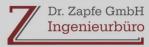
Timber

- + cost-saving for self-mounting
- durability
- contour accuracy









5. Foundation concepts for ground mounted Systems

Pile-driven posts

Screw foundations

Concrete foundation







- •Pull-out capacity (vertical)
- Horizontal stiffness
- •Bending moment in posts
- •Drilling in case of rocks
- •Chemical composition (corrosion)

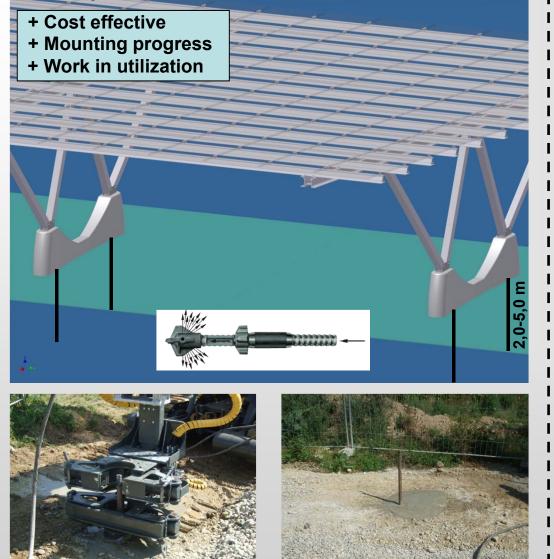
- Pull-out capacity
 - •No horizontal stiffness
- Axial forces
 - •Drilling in case of rocks
- •Chemical composition (corrosion)

- •Pressure stability of the soil
- •Sensitivity of the top soil towards water
- •Aggressive soil





Foundation concepts for Carport Systems (Park@Sol) **Concrete foundations Micro piles**





- + Possible for most soil conditions
- **Cost intensive (material)** -
- Mounting progress _
- Damage of existing parking lots -







6. Mounting progress of different foundation concepts

Ground mounted Systems





Ram-driven posts Co

1 MW/week







Micropiles 1 MW/2 weeks excluding corrugated sheets

1 MW Germany

1 MW/6 weeks including corrugated sheets

Concrete



Carportsystem Park@Sol

6 MW Italy







7. Conclusions

- Design calculations according to national standards
- Safety standards have to be verified for
 - Authorities
 - Insurance
 - Banking
- Target: Minimum BOS costs
 - Material cost
 - Mounting effort
 - Maintenace over life time
- Ram systems can be mounted significant faster
- The suitable system depends on soil contitions

